PROPOSED DEVELOPMENT No.60, 62 & 64 SHOWGROUND ROAD, GOSFORD STORMWATER & WATER CYCLE MANAGEMENT REPORT

	LEGEND					
	DENOTES ON-SITE DETENTION TANK					
	DENOTES ON-SITE RETENTION TANK					
	DENOTES DWELLING FOOTPRINT					
	DENOTES 100mm DIA. STORMWATER/SURFACE WATER SYSTEM PIPE AT 1% MIN. GRADE U.N.O.					
	DENOTES 100mm DIA. FULLY SEALED RAINWATER SYSTEM PIPE U.N.O.					
150 RW	DENOTES RAINWATER PIPE AND DIA. WHEN PIPE EXCEEDS 100mm DIA.					
150	DENOTES STORMWATER/SURFACE WATER PIPE AND DIA. WHEN PIPE EXCEEDS 100mm DIA.					
65 W/RM	DENOTES RISING MAIN AND PIPE DIA. U.N.O.					
100	DENOTES SUBSOIL DRAINAGE LINE AND DIA. WRAPPED IN GEOFABRIC U.N.O.					
DP IO O	DENOTES DOWNPIPE					
	DENOTES INSPECTION OPENING WITH SCREW DOWN LID AT FINISHED SURFACE LEVEL					
çõ	DENOTES INSPECTION OPENING WITH SCREW DOWN LID AT FINISHED SURFACE LEVEL FOR SYSTEM FLUSHING PURPOSES					
	STORMWATER PIT - SOLID COVER					
	STORMWATER PIT - GRATED INLET					
	DENOTES GRATED DRAIN					
	DENOTES ABSORPTION TRENCH					
И	NON RETURN VALVE					
D	PUMP					
M M	STOP VALVE (ISOLATION VALVE)					
¥	240v REQUIRED					
IL23.31	DENOTES LEVEL OF INLET /OUTLET OF STORMWATER PIPE. NOTE: UNLESS NOTED OTHERWISE, THE BASE OF THE PIT IS THE SAME AS THE PIPE INLET/OUTLET.					
DIAL	BEFORE YOU DIG					
	IMPORTANT: THE CONTRACTOR					



IMPORTANT: THE CONTRACTOR
IS TO MAINTAIN A CURRENT SET
OF "DIAL BEFORE YOU DIG"
DRAWINGS ON SITE AT ALL
TIMES

	STORWIVATER & WATER				
	GENERAL NOTES		RAINWATER RE-USE SYSTEM NO	DTES	
1.	THESE PLANS SHALL BE READ IN CONJUNCTION WITH OTHER RELEVANT CONSULTANTS' PLANS, SPECIFICATIONS, CONDITIONS OF DEVELOPMENT CONSENT AND CONSTRUCTION CERTIFICATE REQUIREMENTS. WHERE DISCREPANCIES ARE FOUND ACOR CONSULTANTS (CC) MUST BE CONTACTED IMMEDIATELY FOR	1. 2.	RAINWATER SUPPLY PLUMBING TO BE CONNECTED WHERE REQUIRED BY BASIX CERTIFICATE (BY OTHE TOWN WATER CONNECTION TO RAINWATER TANK TO SATISFACTION OF THE REGULATORY AUTHORITY. REQUIRE PROVISION OF:	RS) O BE TO THE	
2.	VERIFICATION WHERE THESE PLANS ARE NOTED FOR DEVELOPMENT APPLICATION PURPOSES ONLY, THEY SHALL NOT BE USED FOR OBTAINING A CONSTRUCTION CERTIFICATE NOR USED FOR CONSTRUCTION PURPOSES	3.	2.1. PERMANENT AIR GAP 2.2. BACKFLOW PREVENTION DEVICE NO DIRECT CONNECTION BETWEEN TOWN WATER S RAIN WATER SUPPLY	UPPLY AND THE	
3.	SUBSOIL DRAINAGE SHALL BE DESIGNED AND DETAILED BY THE STRUCTURAL ENGINEER. SUBSOIL DRAINAGE SHALL NOT BE CONNECTED INTO THE STORMWATER SYSTEM IDENTIFIED ON THESE	4.	AN APPROVED STOP VALVE AND/OR PRESSURE LIMI THE RAINWATER TANK PROVIDE AT LEAST ONE EXTERNAL HOSE COCK ON		
	PLANS UNLESS APPROVED BY ACOR CONSULTANTS (CC)	5.	WATER SUPPLY FOR FIRE FIGHTING	THE TOWN	
	STORMWATER CONSTRUCTION NOTES	6.	PROVIDE APPROPRIATE FLOAT VALVES AND/OR SOL TO CONTROL TOWN WATER SUPPLY INLET TO TANK ACHIEVE THE TOP-UP INDICATED ON THE TYPICAL D	IN ORDER TO	
1.	ALL WORK SHALL BE CARRIED OUT IN ACCORDANCE WITH AS/NZS 3500 (CURRENT EDITION) AND THE REQUIREMENTS OF THE LOCAL COUNCIL'S POLICIES AND CODES	7.	ALL PLUMBING WORKS ARE TO BE CARRIED OUT BY PLUMBERS IN ACCORDANCE WITH AS/NZS3500.1 NAT AND DRAINAGE CODE		
2.	THE MINIMUM SIZES OF THE STORMWATER DRAINS SHALL NOT BE LESS THAN DN90 FOR CLASS 1 BUILDINGS AND DN100 FOR OTHER CLASSES OF BUILDING OR AS REQUIRED BY THE REGULATORY AUTHORITY	8.	PRESSURE PUMP ELECTRICAL CONNECTION TO BE C A LICENSED ELECTRICIAN		
3.	AUTHORITY THE MINIMUM GRADIENT OF STORMWATER DRAINS SHALL BE 1%, UNLESS NOTED OTHERWISE	9.	ONLY ROOF RUN-OFF IS TO BE DIRECTED TO THE RAINWATER TA SURFACE WATER INLETS ARE NOT TO BE CONNECTED). PIPE MATERIALS FOR RAINWATER SUPPLY PLUMBING ARE TO BE		
4.			APPROVED MATERIALS TO AS/NZS3500 PART 1 SECT CLEARLY AND PERMANENTLY IDENTIFIED AS 'RAINW BE ACHIEVED FOR BELOW GROUND PIPES USING IDI TAPE (MADE IN ACCORDANCE WITH AS2648) OR FOR PIPES BY USING ADHESIVE PIPE MARKERS (MADE IN	TION 2 AND TO BE ATER'. THIS MAY ENTIFICATION ABOVE GROUND	
5.	PUBLIC UTILITY SERVICES ARE TO BE ADJUSTED AS NECESSARY AT THE CLIENT'S EXPENSE		WITH AS1345)		
6.	ALL PITS TO BE BENCHED AND STREAMLINED. PROVIDE STEP IRONS FOR ALL PITS OVER 1.2m DEEP	11.	EVERY RAINWATER SUPPLY OUTLET POINT AND THE TANK ARE TO BE LABELED 'RAINWATER' ON A METAL ACCORDANCE WITH AS1319		
7.	MAKE SMOOTH JUNCTION WITH ALL EXISTING WORK	12.	ALL INLETS AND OUTLETS TO THE RAINWATER TANK SUITABLE MEASURES PROVIDED TO PREVENT MOSO		
8.	VEHICULAR ACCESS AND ALL SERVICES TO BE MAINTAINED AT ALL TIMES TO ADJOINING PROPERTIES AFFECTED BY CONSTRUCTION		VERMIN ENTRY		
9.	SERVICES SHOWN ON THESE PLANS HAVE BEEN LOCATED FROM INFORMATION SUPPLIED BY THE RELEVANT AUTHORITIES AND FIELD		SHEET INDEX		
	INVESTIGATIONS AND ARE NOT GUARANTEED COMPLETE NOR CORRECT. IT IS THE CLIENT & CONTRACTOR'S RESPONSIBILITY TO	СС	VER SHEET & NOTES	SHEET C1	
	LOCATE ALL PRIOR TO CONSTRUCTION	ST	ORMWATER MANAGEMENT PLAN - BASEMENT 4	SHEET C2	
10.	ANY VARIATION TO THE WORKS AS SHOWN ON THE APPROVED	ST	ORMWATER MANAGEMENT PLAN - BASEMENT 3	SHEET C3	
	DRAWINGS ARE TO BE CONFIRMED BY ACOR CONSULTANTS (CC)	ST	ORMWATER MANAGEMENT PLAN - BASEMENT 2	SHEET C4	
	PRIOR TO THEIR COMMENCEMENT	ST	ORMWATER MANAGEMENT PLAN - BASEMENT 1	SHEET C5	
		ST	ORMWATER MANAGEMENT PLAN - GROUND FLOOR	SHEET C6	
		ST	ORMWATER MANAGEMENT DETAILS SHEET No.1	SHEET C7	
		ST	ORMWATER MANAGEMENT DETAILS SHEET No.2	SHEET C8	
		NC	TUSED	SHEET C9	
		ON	-SITE DETENTION REPORT	SHEET C10	
		W	TER QUALITY REPORT SHEET 1	SHEET C11	
		W	ATER QUALITY REPORT SHEET 1	SHEET C12	
		W	TER QUALITY REPORT SHEET 1	SHEET C13	
		FL	DOD SUMMARY	SHEET C14	

ACOR Consultants (CC) Pty Ltd Platinum Building, Suite 2.01, 4 Ilya Avenue ERINA NSW 2250, Australia PROPOSED COMMERCIAL T +61 2 4324 3499 DEVELOPMENT 2 @ @ No.60, 62 & 64 SHOWGROUND ROAD

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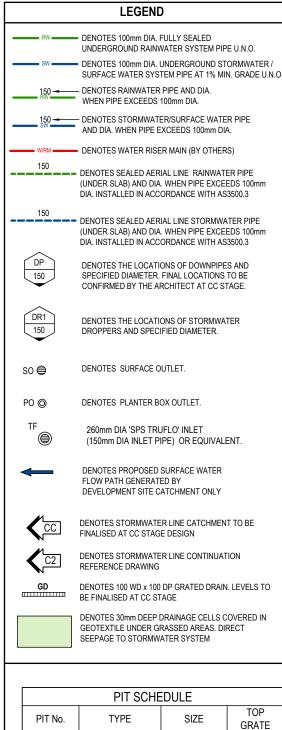
CONSULTANTS ENGINEERS MANAGERS INFRASTRUCTURE PLANNERS DEVEL

GOSFORD

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	CENTRAL C	OAS	г со	UNC	IL RE	QUIR	EM	ENT	S	
tota Proi Proi	AREA (m²) AL PROPOSED IMPER\ POSED ROOF AREA (r POSED DRIVEWAY >. PAVING / COURTYAF	n²)		·				243 233 138 30 960	70 (97%) 30)
GOSI DEVE 6.	POSED COMMERCIAL FORD CITY COUNCIL E ELOPMENT TARGETS / 7.6.1 Intent I developments that require c	OCP 20' ARE RE	3 CLA QUIRE	USE 6 ED.	.7.6.1 TI	HE FOLI	LOWI	NG		
	able 1.				rol Targ				ingoto in	
	Table I	Deve	opmer		/elopment		IX.			
	Development Control Targets	Pools & Spas	Alterations & Additions in excess of 50m ²	Single Dwellings & Dual Occupancy	Medium and High Density Residential Development	Group homes, seniors housing, emergency facilities	Commercial, Industrial	Subdivisions (Urban & Rural)		
	Water Conservation		-	overed b		1	~	x		
	Retention Stormwater Quality	×	×	× ×	*	~	1	× ×		
	Stormwater Quality Onsite Detention	x	x	×	×	~	~	~		
	Local Overland Drainage	~	1	~	~	~	~	~		
	Flooding	~	~	~	~	~	~	1		
REQI PART THE THIS TOTA -	RMWATER RETENTION JIREMENTS OF GOSFI f 6.7.7.2.4 DEEMED TO FOLLOWING MINIMUM REQUIREMENT:- AL RETENTION VOLUM RAINWATER TANK/ REFER WATER QU/ NOTE THIS MEETS THE GET.	ORD DE O COMP RETEN E REQI S = 82,0 ALITY R	EVELO LY. ITION JIRED 100 LIT EPOR	PMEN STOR/ = 82,0 RES T SHE	T CONT AGE IS ⁻ 00 LITR ET C11	ROL PL TO BE F ES	AN 2	013 IDED T	O SATI	
(ii)	ON SITE DETENTIO	N								
REFE	R TO SHEET C10 'ON	SITE DI	ETENT	ION R	EPORT'					
(iii)	STORMWATER QU									
				IEETS	C11 TO	C13				
(iv)	FLOOD PLANNING I			· C14						
	GN PREPARED IN ACC			••••					200 200	12
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COVER SHEET & NOTES					
Drawn SJ	Date 05.07.22	Scale A1 AS NOTED	Q.A. Check	Date -	
Designed BK	Project No. CC220233		Dwg. No. C1	Issue B	



BP1

BP2

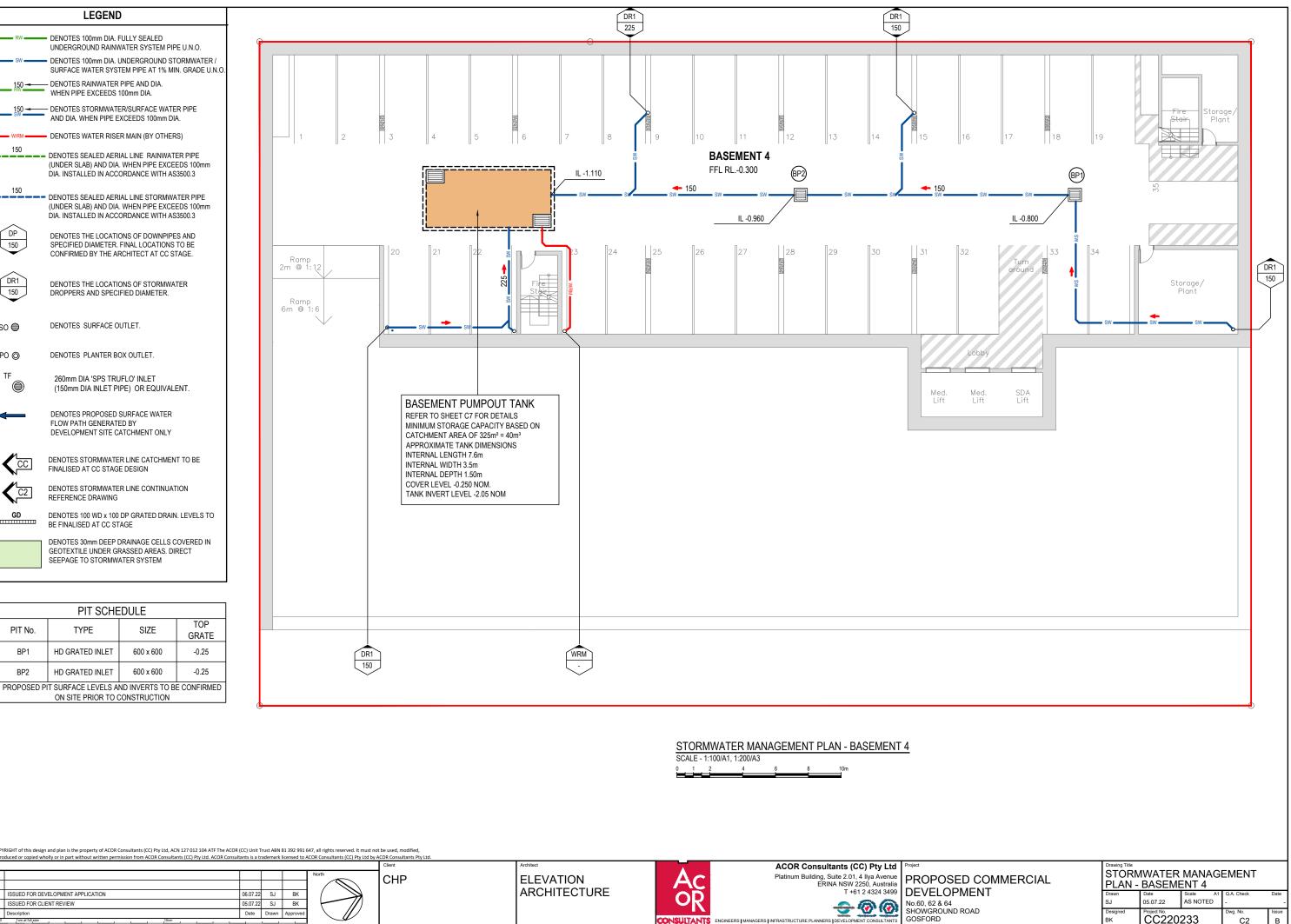
HD GRATED INLET

HD GRATED INLET

ON SITE PRIOR TO CONSTRUCTION

600 x 600

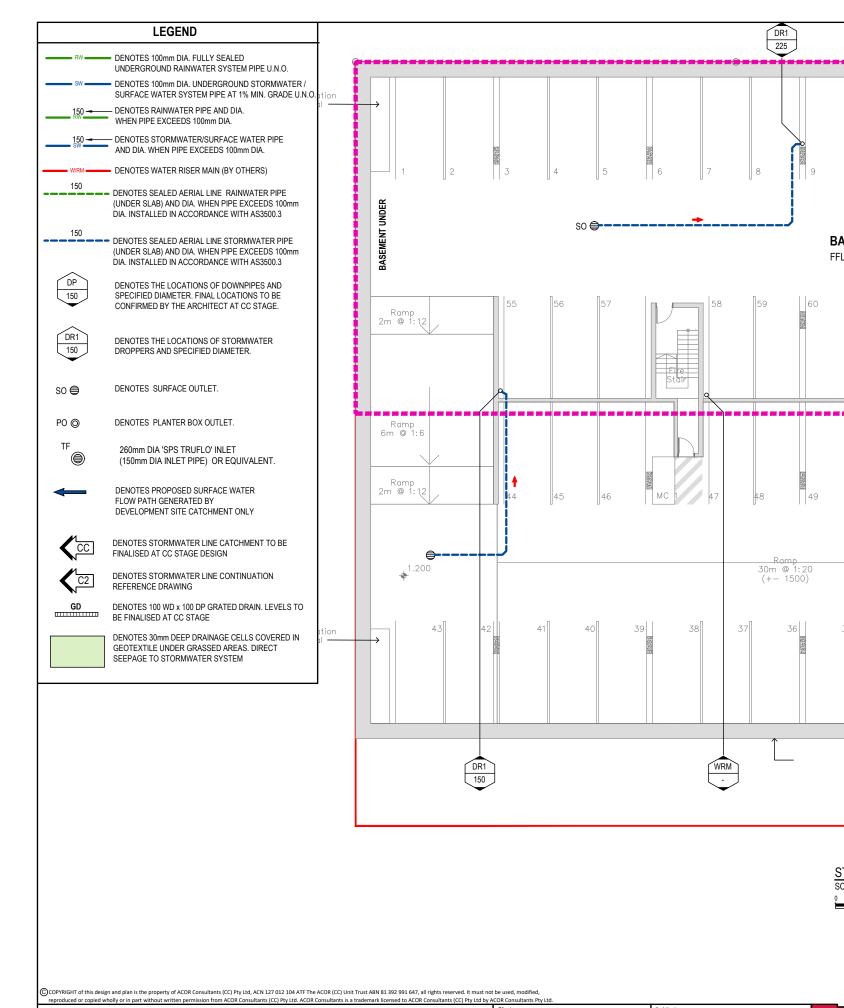
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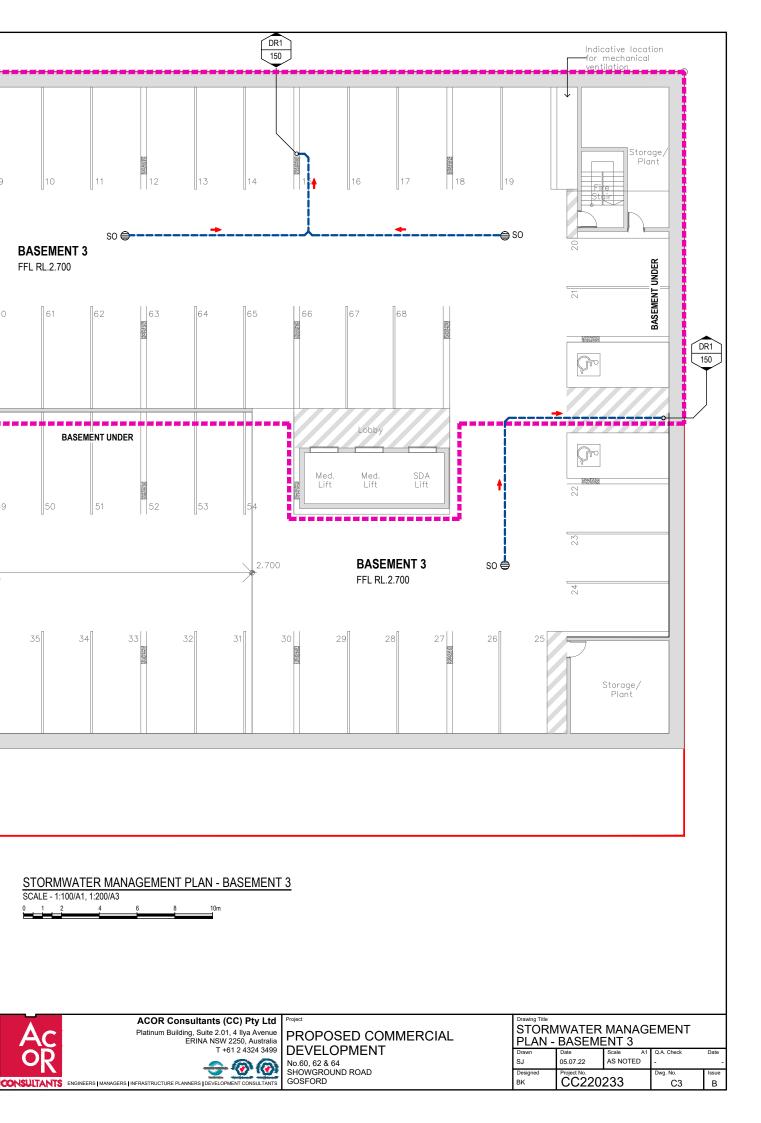
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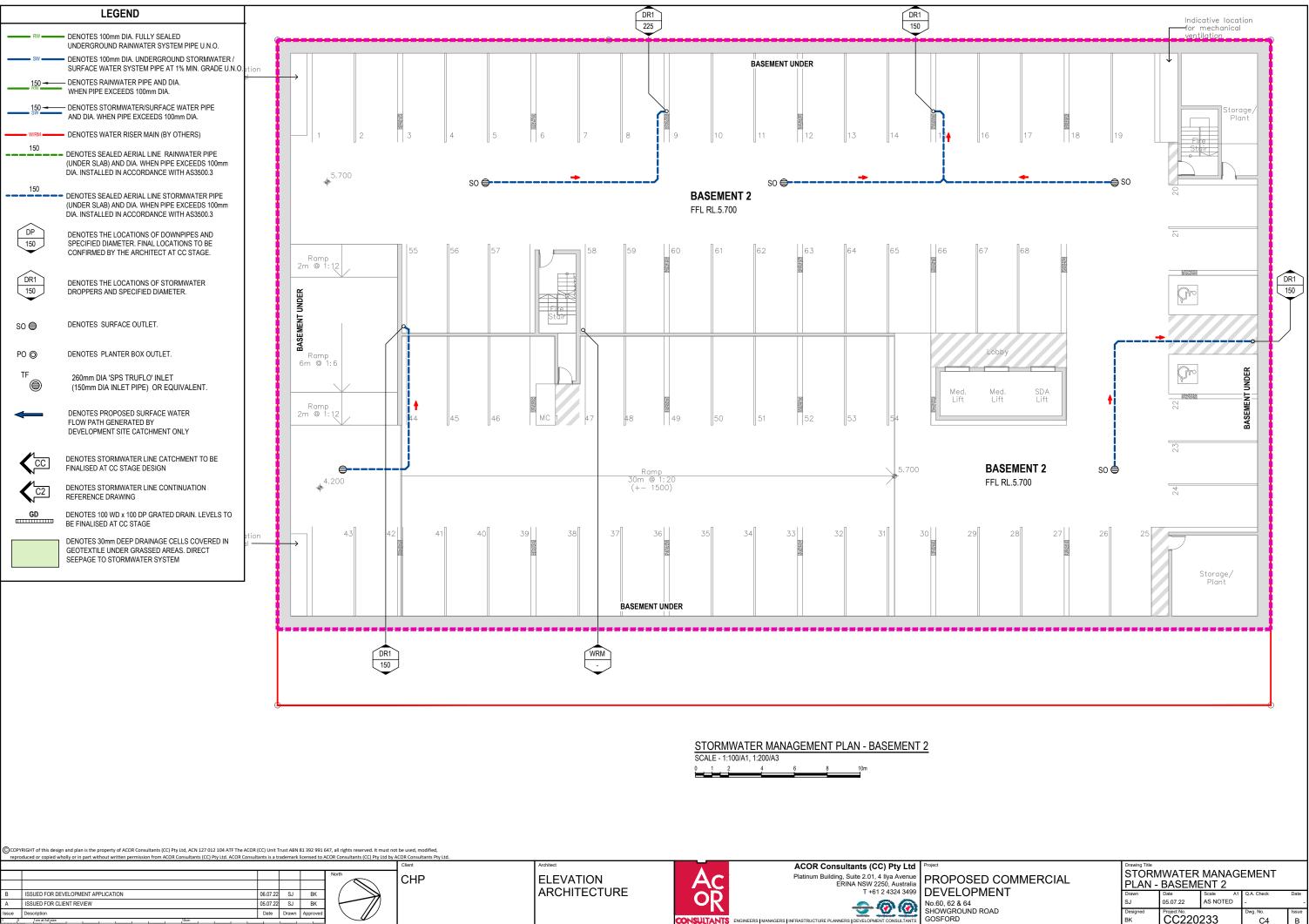




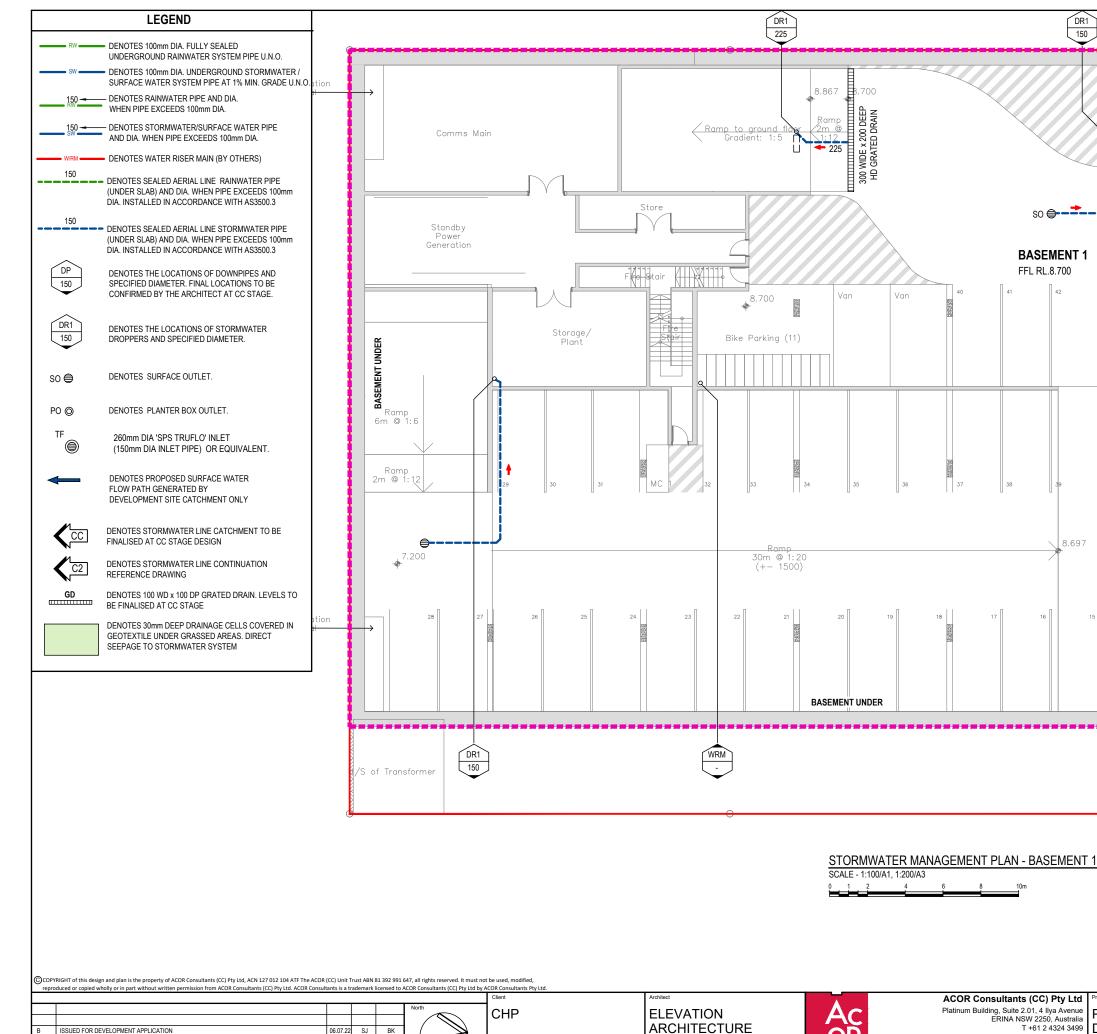
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BASEMENT 1

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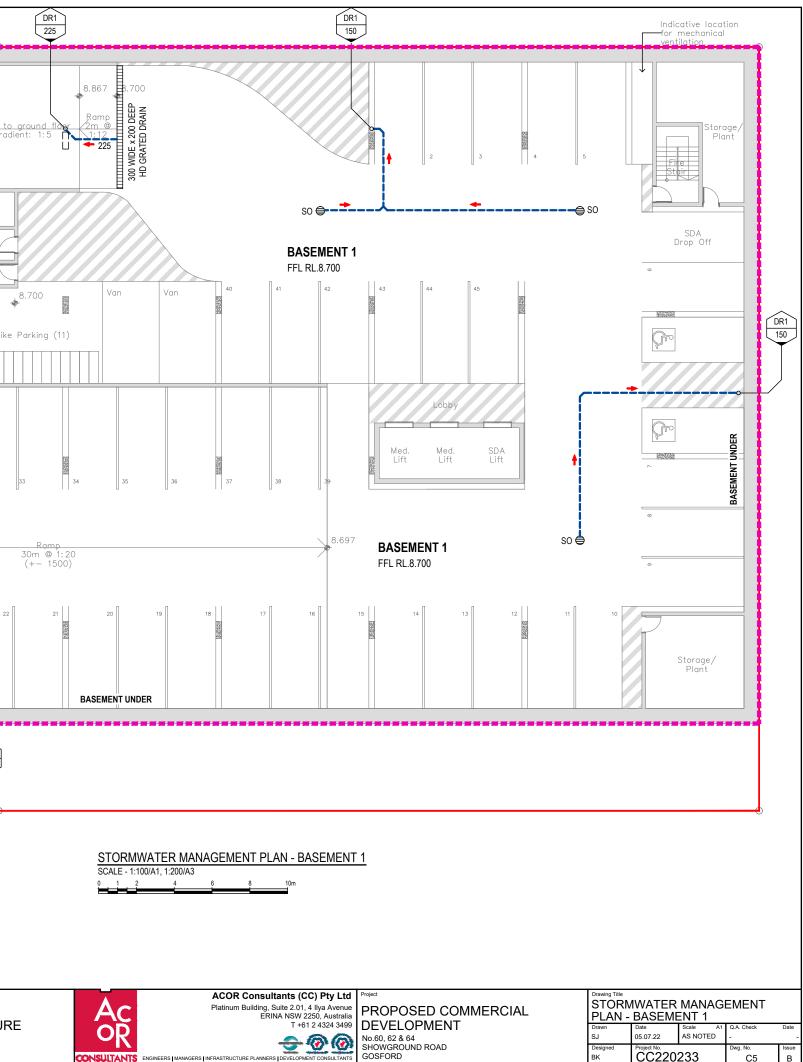
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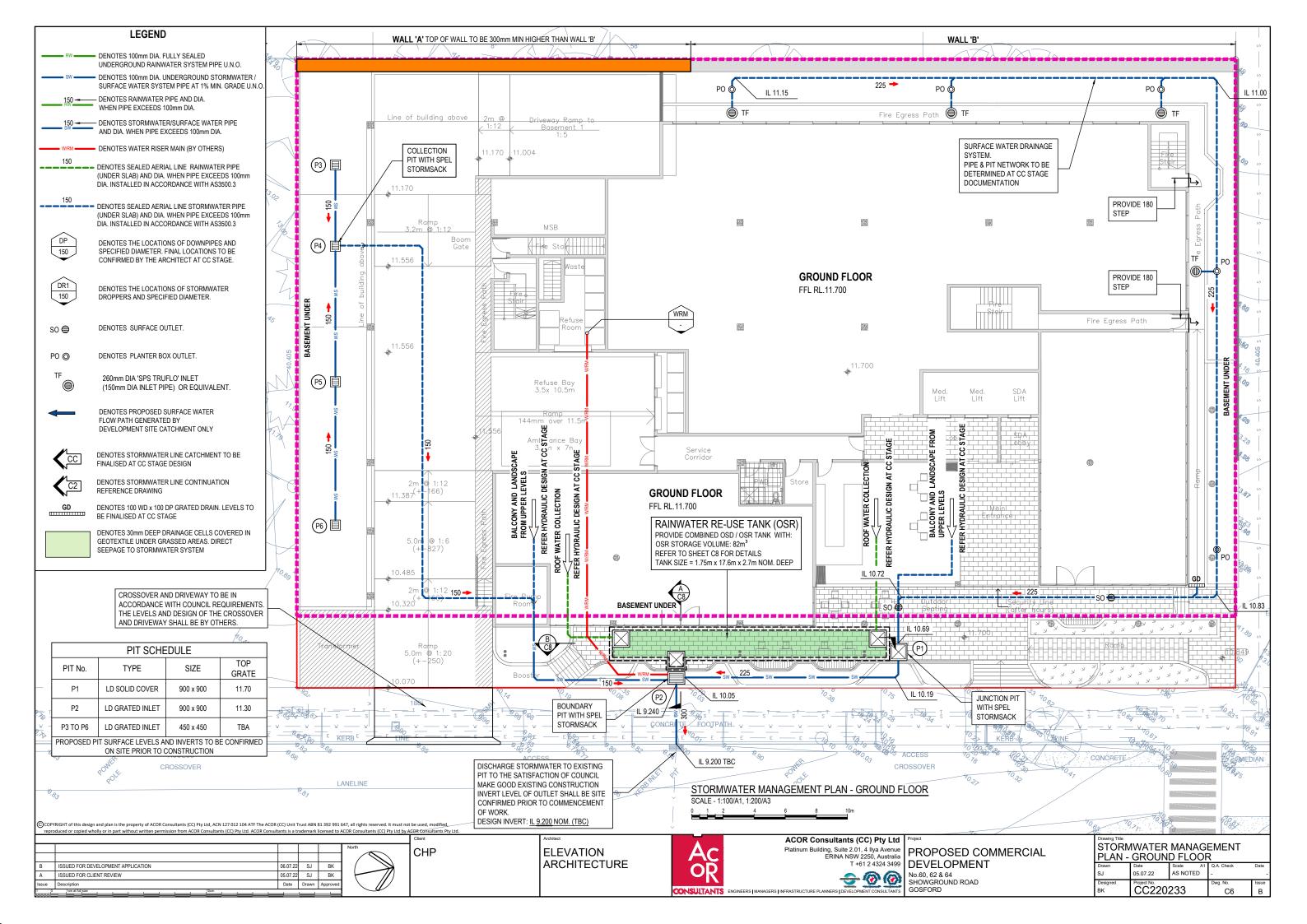
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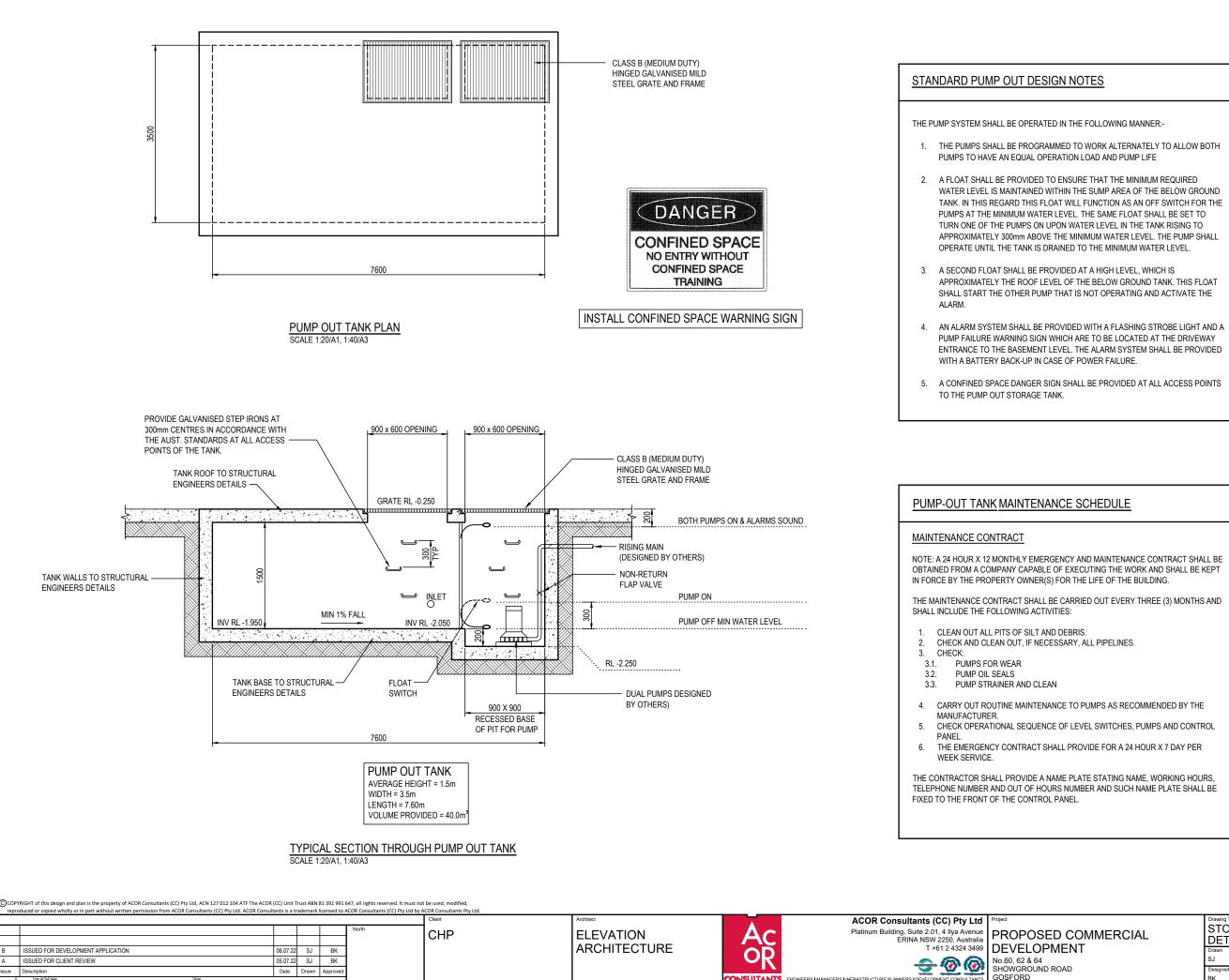
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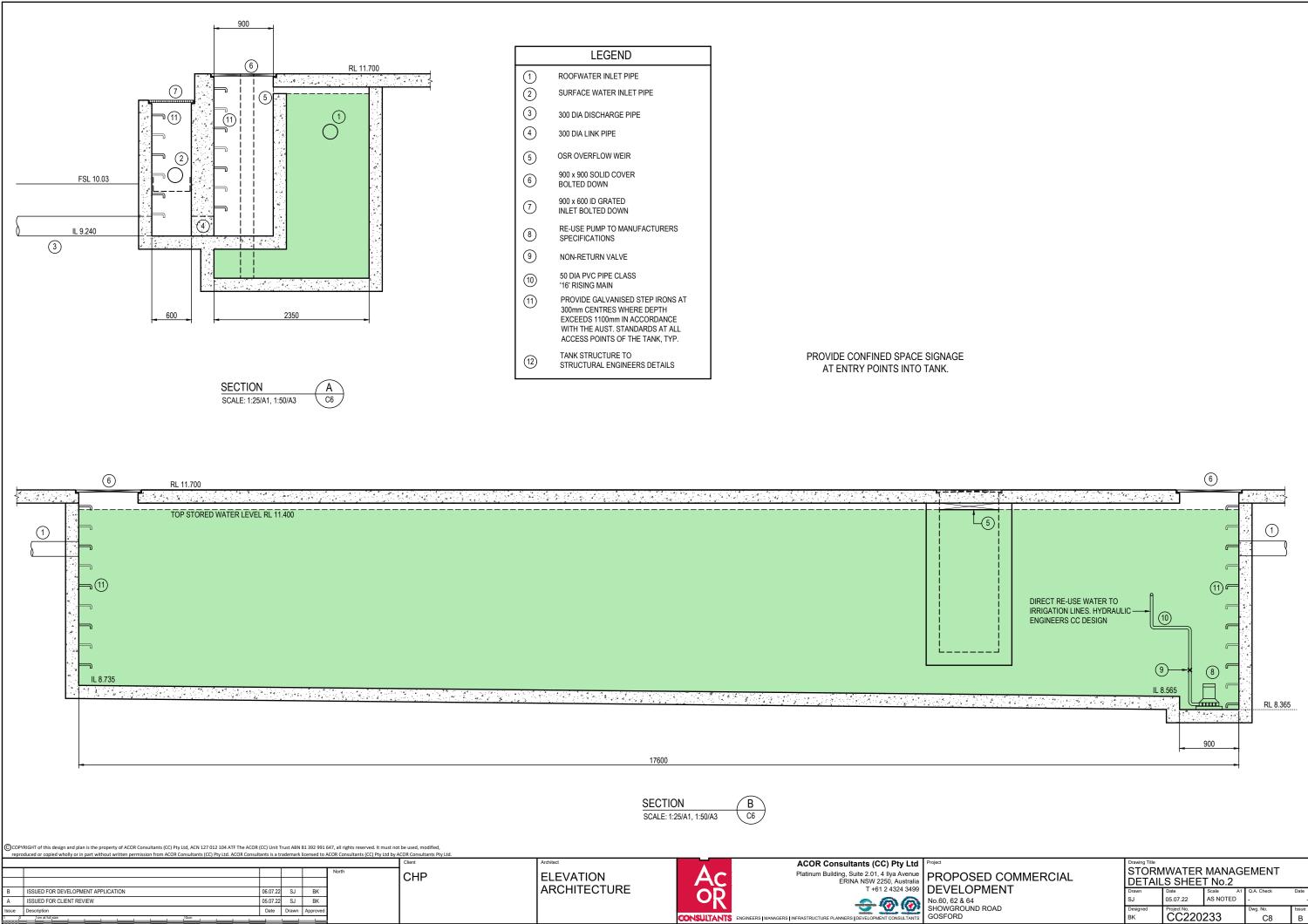






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ON-SITE STORMWATER DETENTION REPORT

1.1. METHODOLOGY

1.1.1. THE DRAINS PROGRAM WAS ADOPTED AS AN APPROPRIATE MODEL FOR THIS PROJECT. PRE-DEVELOPED AND POST-DEVELOPED HYDROLOGICAL AND HYDRAULIC MODELS WERE DEVELOPED FOR THE 1, 2, 5, 10, 20, 50 AND 100 YEAR ARI DESIGN STORM EVENTS, ASSESSING STACKED RAINFALL PATTERNS RANGING FROM 5 MINUTES TO 2 HOURS. THE ADOPTED PRE & POST DEVELOPED FLOWS ARE THOSE ASSIGNED TO THEIR RESPECTIVE PEAKS.

1.2. PRE-DEVELOPED DRAINS MODEL

- 1.2.1. THE PRE-DEVELOPED DRAINS MODEL COMPRISED A SINGLE SUB-CATCHMENT DISCHARGING TO A DUMMY NODE. THE PARAMETERS INPUT TO THE DRAINS MODEL FOR THE SUB-CATCHMENT ARE IDENTIFIED IN THE DRAINS SUB-CATCHMENT DATA INPUT FILE. REFER TO DRAINS FILE "CC220233.dm" THE CATCHMENT AREA ADOPTED IS 0.2438ha. THE PRE & POST DEVELOPED IMPERVIOUS AREAS ADOPTED IN THE MODEL ARE 0% AND 90% RESPECTIVELY.
- 1.2.2. THE PRE-DEVELOPED PEAK FLOWRATES CALCULATED BY THE DRAINS PROGRAM ARE SUMMARISED BELOW:

SITE AREA (m ²)	2438 (0% IMPERVIOUS)
ARI (YEARS)	PEAK FLOWRATE (PRE-DEVELOPED) (L/s)
1	36
2	56
5	76
10	88
20	103
50	113
100	128

1.3. POST-DEVELOPED MODEL

- 1.3.1. THE POST DEVELOPED DRAINS MODEL COMPRISES OF ONE SUB CATCHMENT FORMED BY THE POST DEVELOPED ROOF AREA WHICH DRAINS TO COMBINED OSD / OSR TANKS. REFER TO DRAINS MODEL "CC220233.drn" FOR DETAIL.
- 1.3.2. THE PARAMETERS INPUT INTO THE DRAINS MODEL FOR THE POST-DEVELOPED DETENTION TANKS ARE IDENTIFIED IN THE DRAINS SUB-CATCHMENT DATA. REFER TO DRAINS MODEL "CC220233.drn" FOR DETAILS.
- 1.3.3 THE OSD STORAGE/OUTFLOW PARAMETERS ADOPTED IN THE DRAINS MODEL ARE IDENTIFIED IN DRAINS MODEL "CC220233.drn"
- 1.3.4 THE PEAK STORAGE VOLUME CALCULATED BY THE DRAINS MODEL OCCURS DURING THE 100 YEAR ARI 25 MINUTE DESIGN STORM EVENT. THE VOLUMETRIC GRAPH FOR THIS STORM EVENT IS IDENTIFIED IN DRAINS MODEL "CC220233.drn".

1.3. POST-DEVELOPED MODEL (CONTINUED)

- 1.3.5. THE INFLOW AND OUTFLOW HYDROGRAPH FOR THIS STORM EVENT IS IDENTIFIED IN DRAINS MODEL "CC220233.dm
- THE PEAK FLOWRATES AND WATER SURFACE LEVELS DEVELOPED 1.3.6. BY THE DRAINS MODEL FOR THE 100 YEAR ARI DESIGN STORM EVENT. REFER TO DRAINS MODEL "CC220233.drn" FOR DETAIL

ARI (YEARS)	PRE - DEVELOPED FLOW RATE (L/s)	POST - DEVELOPED TOTAL FLOW RATE (L/s)	STORAGE VOLUME (m ³)
1	36	47	6
2	56	53	10
5	76	60	19
10	88	65	24
20	103	70	28
50	113	74	36
100	128	114	41

1.5 CONCLUSION

1.1.6.

IN ACCORDANCE WITH CENTRAL COAST COUNCIL DCP SECTION 6.7.7.4.4, THE OSD REQUIREMENT OF 41 m³ HAS BEEN OFFSET BY 50% OF THE RAINWATER RE-USE TANK PROVIDED. IN THIS REGARD 82 m³ RAINWATER RE-USE IS PROPOSED AND SUBSEQUENTLY THE OSD REQUIREMENT IS OFFSET ENTIRELY BY THE PROVISION OF THE RAINWATER TANK.

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Issue	Description	Date	Drawn	Approved		
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BASED ON THE FOREGOING AN OSD TANK OF 41 m³ WILL ATTENUATE POST-DEVELOPED PEAK FLOWRATES TO EQUIVALENT FLOWRATES OR LESS THAN THE COMPARABLE PRE-DEVELOPED FLOWRATES. THE PEAK FLOWRATES FOR THE PRE & POST-DEVELOPED STORM EVENTS FOR THE ENTIRE CATCHMENT DISCHARGE TO THE EXISTING STORMWATER SYSTEM.

DMMERCIAL	STORMWATER DETENTION REPORT					
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	SJ	05.07.22	AS NOTED	-	-	
	Designed Project No.			Dwg. No.	Issue	
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WATER QUALITY REPORT

INTRODUCTION

A CATCHMENT BASED WATER QUALITY MODEL WAS DEVELOPED TO INVESTIGATE STORMWATER RUNOFF QUALITY FROM THE SUBJECT SITE IN ACCORDANCE WITH GOSFORD CITY COUNCIL'S DEVELOPMENT CONTROL PLAN 2013 PART 6.7 "WATER CYCLE MANAGEMENT." THE REQUIREMENTS ARE TABLED FOLLOWING AS EXTRACTED FROM CLAUSE 6.7.7.3.2:

POLLUTANT	% RETENTION OF THE ANNUAL AVERAGE LOAD (kg/ha/yr)
GROSS POLLUTANTS	90%
TOTAL SUSPENDED SOLIDS	80%
TOTAL PHOSPHORUS	45%
TOTAL NITROGEN	45%

ON - SITE RETENTION TARGET 1.1

THE TABLE BELOW IDENTIFIES THE REQUIRED STORMWATER RETENTION TARGET UNDER THE GOSFORD CITY COUNCIL DCP 2013.

Table 2 Stormwater Retention Volume Target (m³)

50000											
00000	0	20	80	180	320	500	720	980	1280	1620	2000
10000	0	4.0	16	36	64	100	144	196	256	324	400
5000	0	2.0	8.0	18	32	50	72	98	128	162	200
2000	0	0.8	3.2	7.2	13	20	29	39	51	65	80
1500	0	0.6	2.4	5.4	9.6	15	22	29	38	49	60
1000	0	0.4	1.6	3.6	6.4	10	14	20	26	32	40
900	0	0.4	1.4	3.2	5.8	9.0	13	18	23	29	36
800	0	0.3	1.3	2.9	5.1	8.0	12	16	20	26	32
700	0	0.3	1.1	2.5	4.5	7.0	10	14	18	23	28
600	0	0.2	1.0	2.2	3.8	6.0	8.6	12	15	19	24
500	0	0.2	0.8	1.8	3.2	5.0	7.2	9.8	13	16	20
400	0	0.2	0.6	1.4	2.6	4.0	5.8	7.8	10	13	16
	0	10	20	30	40	50	60	70	80	90	100

Fraction Impervious (%)

RESPONSE: TOTAL RETENTION REQUIRED FOR SITE AREA OF 2438 m² WHICH IS 90% IMPERVIOUS EQUALS 79 m³ TOTAL RETENTION PROVIDED FROM RAIN WATER TANK (82 m³)

STUDY METHODOLOGY 2.

THE OBJECTIVES OF THIS REPORT ARE TO:

ASSESS THE STORMWATER QUALITY FOR THE POST DEVELOPMENT SCENARIO AND PROVIDE RECOMMENDATIONS TO ENSURE THE DEVELOPMENT MEETS FLOOD RUNOFF QUALITY STANDARDS WHERE REQUIRED.

THE REPORT IS BASED ON THE APPLICATION OF MUSIC (MODEL FOR URBAN STORMWATER IMPROVEMENT CONCEPTUALISATION) MODELLING WHICH INCLUDED THE FOLLOWING:

- A STORMWATER QUALITY MODEL TO CONVERT RAINFALL AND EVAPOTRANSPIRATION ON THE CATCHMENT INTO RUNOFF.
- ESTIMATE STORMWATER FLOW AND POLLUTION GENERATION BY SIMULATING THE PERFORMANCE OF STORMWATER TREATMENT DEVICES INDIVIDUALLY AND AS PART OF A TREATMENT TRAIN.

THE MODEL DEFINES WATER QUALITY PROFILES FOR THE POST DEVELOPED TREATED AND UNTREATED SCENARIOS. THE TREATED POST DEVELOPED MODEL INCLUDES POLLUTANT REDUCTION PERCENTAGES, WHICH REFLECT WORKS THAT ARE ESSENTIAL TO MEET THE RELEVANT REQUIREMENTS SCRIBED BY COUNCIL FOR A PROJECT OF THIS NATURE.

RAINFALL AND EVAPOTRANSPIRATION DATA 3.

FOR THE PURPOSE OF THIS REPORT DATA HAS BEEN OBTAINED FROM CENTRAL COAST COUNCIL'S MUSIC LINK VERSION 6.34 FOR A SITE LOCATED WITHIN THE LOWLAND REGION.

STORMWATER QUALITY MODELLING 4.

4.1 GENERAL

THE FOLLOWING PARAMETERS WERE ASSESSED IN THE HYDROLOGICAL MODELLING ASSOCIATED WITH THE CATCHMENT.

- RAINFALL/RUNOFF AND EVAPOTRANSPIRATION.
- SUB CATCHMENT DIVERSIONS.
- LAND USE (PERVIOUS AND IMPERVIOUS)

4.2 RAINFALL/RUNOFF AND EVAPOTRANSPIRATION

THE DEFAULT MONTHLY AVERAGE POTENTIAL EVAPOTRANSPIRATION PROVIDED BY CENTRAL COAST COUNCIL'S MUSIC LINK VERSION 6.34 WAS UTILISED IN THIS STUDY.

THE DETAILS ARE SUMMARISED IN TABLE 4.1 AND 4.2 FOLLOWING:

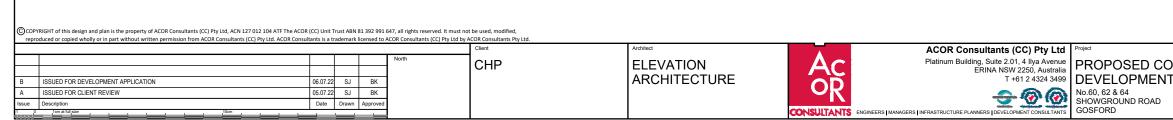


TABLE 4.1 - DETAILS OF DAILY RAINFALL DATA						
STATION NAME PERIOD TIMESTEP						
066062	SYDNEY OBSERVATORY HILL	01/01/1974-01/01/1994	6 min			

TABLE 4.2 - SUMMARY OF POTENTIAL EVAPOTRANSPIRATION (PET)								
JAN	FEB	MAR	APR	MAY	JUN			
180.11	134.96	128.03	84.90	57.97	42.90			
JUL	AUG	SEP	OCT	NOV	DEC			
43.09	57.97	87.90	127.10	152.10	163.06			

CATCHMENT DEFINITION 4.3

THE CATCHMENT AREA UNDER POST DEVELOPMENT SCENARIO IS DIVIDED INTO THREE (4) SUB-CATCHMENTS, WHICH WERE DEFINED BASED ON FUNCTIONAL AREAS AND ANTICIPATED OVERLAND FLOW PATHS. THE DETAILS OF THE SUB-CATCHMENTS ARE SUMMARISED IN FOLLOWING TABLE 4.3.

TABLE 4.3 - POST DEVELOPMENT SUB CATCHMENT DETAILS							
SUB CATCHMENT ID	SUB CATCHMENT AREA (ha)	% IMPERVIOUS AREA	% PERVIOUS AREA				
COMBINED ROOF TO RAINWATER TANK	0.138	100	0				
FOOTPATHS AND LANDSCAPING	0.049	60	40				
OPEN ROOFTOP PLANTER AREAS	0.032	0	100				
ROOFTOP TERRACE AREA	0.025	100	0				

DMMERCIAL WATER QUALITY REPORT SHEET 1							
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	BK	вк СС220233			В		

MUSIC MODEL 5.

THE MUSIC MODEL WAS CREATED BASED ON A 6 min RAINFALL-RUNOFF MODEL IN CONJUNCTION WITH RESENTATIVE BASEFLOW AND STORMFLOW EVENT MEAN CONCENTRATION (EMCs).

WATER QUALITY PARAMETERS 5.1

THE ADOPTED VALUES OF VARIOUS MUSIC RAINFALL AND RUNOFF PARAMETERS ARE SUMMARISED IN TABLE 5.1.

TABLE 5.1 - ADOPTED MUSIC RAINFALL/RUNOFF PARAMETERS					
PARAMETER	VALUE				
IMPERVIOUS AREA PROPERT	TIES				
RAINFALL THRESHOLD (mm/DAY)	1.0				
PERVIOUS AREA PROPERTI	ES				
SOIL STORAGE CAPACITY (mm)	200				
SOIL INITIAL STORAGE (% OF CAPACITY)	30				
FIELD CAPACITY (mm)	80				
INFILTRATION CAPACITY COEFFICIENT - a	200				
INFILTRATION CAPACITY EXPONENT - b	1				
GROUNDWATER PROPERTI	ES				
INITIAL DEPTH (mm)	10				
DAILY RECHARGE RATE (%)	0				
DAILY BASEFLOW RATE (%)	0				
DAILY DEEP SEEPAGE RATE (%)	2.0				

STORMWATER QUALITY IS CHARACTERISED USING EVENT MEAN CONCENTRATION (EMCs) UNDER STORM AND BASE FLOW CONDITIONS. THE VALUE OF WATER QUALITY PARAMETERS ADOPTED IN THIS STUDY IS SUMMARISED IN TABLE 5.2

TABLE 5.2 - ADOPTED MUSIC WATER QUALITY PARAMETERS							
LAND-USE		Log₁₀TSS (mg/L)		Log₁₀TP (mg/L)		Log₀ TN (mg/L)	
CATE	CATEGORY		BASE FLOW	STORM FLOW	BASE FLOW	STORM FLOW	BASE FLOW
GENERAL	MEAN	2.15	1.20	-0.60	-0.85	0.30	0.11
URBAN	STD DEV	0.32	0.17	0.25	0.19	0.19	0.12
	MEAN	2.43	*	-0.3	*	0.34	*
ROADS	STD DEV	0.32	*	0.25	*	0.19	*
	MEAN	1.30	*	-0.89	*	0.30	*
ROOFS	STD DEV	0.32	*	0.25	*	0.19	*

BASE FLOWS ARE ONLY GENERATED FROM PERVIOUS AREAS; THEREFORE THESE PARAMETERS ARE NOT RELEVANT. THEREFORE THESE PARAMETERS ARE NOT RELEVANT.

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1 0	10m tfull size 10cm					

5.2 STORMWATER TREATMENT MEASURES

THE STORMWATER TREATMENT MEASURES THAT WERE ASSESSED USING MUSIC INCLUDED ONE OSR TANK (COMBINED FOR THE DEVELOPMENT) AND TWO SPEL STORMSACK INSERTS OR APPROVED EQUAL. THE CONCEPTUAL PLAN FOR THE PROPERTY IS SHOWN ON SHEET C9. THE ADOPTED WATER QUALITY TREATMENT TRAIN DEVICES ARE LISTED IN TABLE 5.3 AND THE PROPERTIES OF THE RAINWATER TANK AND RE-USE IS SHOWN IN FIGURE 5.1.

TABLE 5.3 - TREATMENT TRAIN DEVICES							
	OSR VOLUME OSD SPEL STORMSAC						
COMBINED ROOF FOR THE DEVELOPMENT	82 kL	OSD OFFSET BY RAINWATER TANK	2 X 600 SQ				

5.3 MODEL DEFINITION

THE MODEL LAYOUT FOR THE AND POST DEVELOPED SCENARIOS IS DEPICTED ON THIS SHEET.

RESULTS & CONCLUSION 6.

BASED ON THE FOREGOING THE PROPOSED NUTRIENT CONTROL MEASURES ACHIEVE THE REQUIRED NUTRIENT REMOVAL TARGET LEVELS. THE RESULTS OF MUSIC MODELLING ARE SUMMARISED IN TABLE 6.1 FOLLOWING. ALSO REFER MUSIC LINK REPORT REFERENCE CC220233 musicLink Report.pdf

TA				
PARAMETER	SOURCE RUNOFF	DISCHARGE FROM SITE	REDUCTION	
POST-D				
FLOW (ML/y)	5.09	5.09	0%	
TSS (kg/y)	675	v	0%	
TP (kg/y)	1.27	1.27	0%	
TN (kg/y)	11.1	11.1	0%	
GROSS POLLUTANTS (kg/y)	54.4	54.4	0%	
	POST-DEVELO	PMENT	-	REDUCTION TARGET
FLOW (ML/y)	5.09	3.66	28.1%	
TSS (kg/y)	675	119	82.4%	80%
TP (kg/y)	1.27	0.513	59.6%	45%
TN (kg/y)	11.1	6.01	45.8%	45%
GROSS POLLUTANTS (kg/y)	54.4	0	100%	90%

Location Rainwater Tank (82 kL)	Products >
Inlet Properties	
Low Flow By-pass (cubic metres per ser	c) 0.000000
High Flow By-pass (cubic metres per se	c) 100.000000
ndividual Tank Properties	
+ Number of Tanks	1
Total Tank Properties	
Storage Properties	
Volume below overflow pipe (kL)	82.00
Depth above overflow (metres)	0.20
Surface Area (square metres)	41.0
Initial Volume (kL)	41.00
Outlet Properties	
Overflow Pipe Diameter (mm)	225
Use Custom Outflow and Storage	Relationship
Define Custom Outflow and Sto	rage Not Defined
Re-use Fluxes No	tes More

FIGURE 5.1 - RAINWATER TANK PROPERTIES

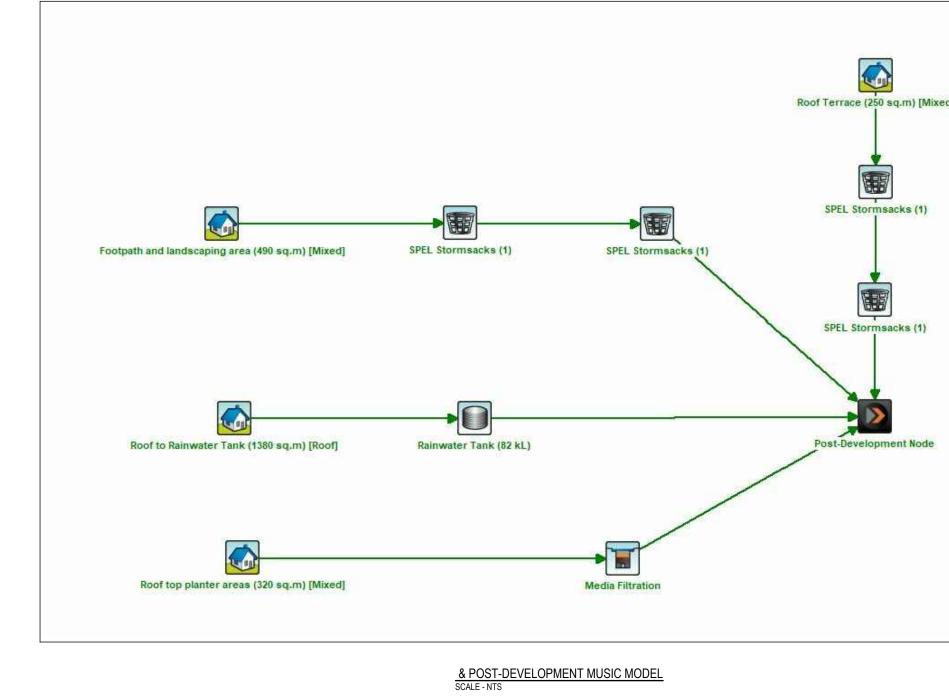
RAINWATER RE-USE HAS BEEN DETERMINED BASED ON ANTICIPATED IRRIGATION USAGE TO SERVICE THE GARDEN AND PLANTER AREAS WITH 20mm WATER PER WEEK.



OR



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FLOODING AND LOCAL OVERLAND DRAINAGE SUMMARY

1.1. LOCAL FLOOD BEHAVIOUR

THE SITE IS IMPACTED BY 1% AEP FLOODWATERS PONDING IN THE LOWPOINT IN SHOWGROUND ROAD. FLOOD BEHAVIOUR IN THE VICINITY OF THE SITE IS DESCRIBED IN 'GOSFORD CBD LOCAL OVERLAND FLOW FLOOD STUDY' PREPARED BY CARDNO, PROJECT No. W4816, VERSION 10, DATED 18 SEPTEMBER 2013. THE SAG WITHIN SHOWGROUND ROAD ADJACENT TO THE SITE HAS BEEN IDENTIFIED IN CARDNO 2013 AS REFERENCE LOCATION GC-1. TABLE A.1 IN CARDNO 2013 PROVIDES A SUMMARY OF PEAK FLOODWATER LEVELS IMPACTING THE SAG IN SHOWGROUND ROAD WHICH ARE APPLICABLE TO THE SUBJECT SITE. THESE LEVELS HAVE BEEN REPRODUCED IN TABLE 1 BELOW.

TABLE 1 - APPLICABLE FLOOD LEVELS AND FLOOD PLANNING LEVELS

FLOOD LEVEL INFORMATION FOR LOCATION GC-1 BASED ON INFORMATION DERIVED FROM GOSFORD CBD OVERLAND FLOW FLOOD STUDY.										
STORM EVENT	FLOOD LEVEL (m AHD)	FLOOD PLANNING LEVEL (COMMERCIAL LAND USE)	FLOOD PLANNING LEVEL (SENSITIVE LAND USE)							
10% AEP	10.10									
5% AEP	10.14									
2% AEP	10.17	10.76 m AHD	11.9 m AHD							
1% AEP	10.26]								
PMF	11.9									

1.2. FLOOD RELATED DEVELOPMENT CONTROLS

1.2.1. THE FLOOD RELATED DEVELOPMENT CONTROLS APPLICABLE TO THE PROPOSED DEVELOPMENT ARE IDENTIFIED IN TABLE 4 - FLOOD CONTROL MATRIX IN CENTRAL COAST DCP 2013 PART 6.7.7.6.

IN THIS REGARD, THE FLOOD PLANNING LEVELS APPLICABLE TO THE PROPOSED DEVELOPMENT ARE LISTED IN TABLE 1 ABOVE.

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OR

ACOR Consultants (CC) Pty Ltd Platinum Building, Suite 2.01, 4 Ilya Avenue ERINA NSW 2250, Australia PROPOSED CO T +61 2 4324 3499 DEVELOPMENT No.60, 62 & 64 SHOWGROUND ROAD __@@ CONSULTANTS ENGINEERS MANAGERS INFRASTRUCTURE PLAN GOSFORD

1.3. PROPOSED FLOOR LEVEL COMPLIANCE

THE APPLICANT PROPOSES A GROUND FLOOR LEVEL COMPRISING COMMERCIAL DEVELOPMENT OF APPROXIMATELY 11.7 m AHD. THIS LEVEL PROVIDES 1.44 m FREEBOARD TO THE 1% AEP FLOOD LEVEL OF RL 10.26 m AHD WITHIN SHOWGROUND ROAD.

THE UPPER FLOOR LEVELS PROPOSED FOR SPECIALIST DISABILITY ACCOMMODATION PROVIDES A MINIMUM HABITBALE FLOOR LEVEL OF RL 16.5 m AHD. THIS LEVEL PROVIDES 4.6 m FREEBOARD TO THE PROBABLE MAXIMUM FLOOD LEVEL OR RL 11.9 m AHD WITHIN SHOWGROUND ROAD.

1.4 FLOOD IMPACTS

WE REFER TO FIGURE 4.21 OF CARDNO 2013 WHICH DEPICTS THE 1% AEP FLOODWATER EXTENTS AND HYDRAULIC CATEGORY WITHIN SHOWGROUND ROAD ADJACENT TO THE SITE. WE NOTE THAT THE EXTENT OF FLOOD STORAGE AREA IS GENERALLY CONTAINED WITHIN THE ROAD RESERVE. BASED ON THE FOREGOING, WE ANTICIPATE THE PROPOSED DEVELOPMENT WILL RESULT IN NEGLIGIBLE LOSS OF FLOOD STORAGE AND RESULT IN NEGLIGIBLE IMPACT TO EXISTING 1% AEP FLOOD BEHAVIOUR WITHIN SHOWGROUND ROAD.

1.5 EVACUATION

WE NOTE THAT THE PROPOSED HABITIABEL FLOORS ARE LOCATED ABOVE THE PMF FLOOD LEVEL OF RL 11.9 m AHD. IN THIS REGARD, OCCUPANTS OF THE PROPOSE DEVELOPMENT ARE ABLE TO REMAIN ON SITE DURING ALL FLOOD EVENTS.

1.6 CONCLUSION

BASED ON THE FOREGOING, WE HAVE FORMED THE VIEW THAT THE PROPOSED DEVELOPMENT WILL NOTE RESULT IN SIGNIFICANT ADVERSE IMPACTS TO EXISTING 1% AEP FLOOD BEHAVIOUR AND GENERALLY COMPLIES WITH THE MINIMUM FLOOR LEVEL REQUIREMENTS OF CENTRAL COAST COUNCIL FOR A DEVELOPMENT OF THIS NATURE.

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